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Editors

Dr Patrick Manu
Mr Tony Westcott
Professor Dr Alan Spedding

EFFECT OF CAMERA CALIBRATION ON THE ACCURATE GENERATION OF DIGITAL ELEVATION MODELS FROM UAV ACQUIRED LOW PERCENTAGE OVERLAPPING IMAGES

Oluibukun Gbenga Ajayi¹, Ifeanyi Jonathan Nwadiolor², Ifeanyi Chukwudi Onuigbo³, and Olurotimi Adebowale Kemiki⁴.

^{1, 2, 3} *Department of Surveying and Geoinformatics, Federal University of Technology, Minna, Nigeria.*

⁴ *Department of Estate Management, Federal University of Technology, Minna, Nigeria.*

Using low percentage overlapping images (15-20% sidelap and endlap) acquired from a DJI Phantom 3 Quadcopter UAV, and a 4K resolution digital camera (FC300X), an attempt has been made in this research to investigate the impact of camera calibration on the accuracy of Digital Elevation Models (DEMs) using four (4) possible case scenarios which are: (1) Processing without calibration, (2) Processing with calibration before optimization of camera alignment, (3) Processing with calibration during optimization of camera alignment and (4) Processing with calibration after optimization of camera alignment. The images were processed using Agisoft photoscan digital photogrammetric software. Eastings, Northings and Height coordinates of ground control points were extracted from the DEM generated based on these four conditions and compared with the coordinates of the same points established using Hi-Target Differential Global Positioning System (DGPS) receivers. Using the National Standard for Spatial Data Accuracy (NSSDA), the result obtained showed that though the impact was quite trivial on the Eastings and Northings coordinates extracted from the DEMs produced under these four camera calibration conditions (each producing a horizontal accuracy of 0.003786) and more evident on the height values, DEM generated by calibrating the camera before optimization of camera alignment yielded the best accuracy (with vertical accuracy of 0.08956) when compared to the other three conditions. It is thus recommended that the operation of camera calibration be performed before the optimization of camera alignment when processing UAV acquired image data for DEM production using Agisoft photoscan digital photogrammetric software.

Keywords: Camera calibration, UAV photogrammetry, 3D Elevation Models, Camera alignment, Calibration parameters, Optimization.

INTRODUCTION

Digital elevation models (DEMs) are vital resources for various applications such as climate impact studies, water and wildlife management, geological and hydrological modelling, geomorphology and landscape analysis, earth modelling etc. (Sulebak, 2000). They can be described as spatially geo-referenced data set, that continuously represent or depict

¹ ogbajayi@gmail.com

² drijnwadiolor@yahoo.com

³ anyi.onuigbo@futminna.edu.ng

⁴ kemiki@futminna.edu.ng

the topography of an area effectively by elevation values and snap shot of land scape features (Isioye and Paul, 2011). They can be produced from aerial photographs (Toz and Erdogan, 2008) and satellite images (Bolstad and Stowe, 1994), etc.

With the gradual influx of technology in virtually all facets of human endeavours, photogrammetry has become more globally developed, migrating from the old age-long analogue method of data acquisition and processing to digital and computerised form. The Unmanned Aerial Vehicle (UAV) has greatly thrived on the wings of technology, providing a high scale, time-effective and low-cost facility for earth monitoring (Fonstad et al., 2013; Sammartano and Spanò, 2016). Its application is fast gaining wide and public acceptance in almost all fields of human endeavours, most especially in physical sciences (Ajayi et al., 2017), and when related more specifically to photogrammetry, it has led to photogrammetry (Birute et al., 2014).

Camera calibration is one of the major steps involved in the photogrammetric processing of overlapping images for the production of DEMs (Weng et al., 1992). It is the process of finding the internal characteristics of an auto camera and finding the camera's location in space with respect to a fixed object. This is very essential when lens distortion is to be corrected or the size of the object in world units is to be measured. Cameras can be calibrated beforehand, after the flight (Anuar, 2011) or through the flight mission (Eisenbeiss, 2009) in order to extract metric information from the 2D images (Zhang, 2004), correct for lens distortion, measure the size of an object in world units, or determine the location of the camera in the scene. Norhadija et al. (2013) also opined that camera calibration is expedient if accurate and reliable measurements or results must be obtained when using UAV for environmental or photogrammetric applications.

Many calibration methods have been developed in the last couple of years (Zhang et al., 2010; Douskos et al., 2007) but most of these methods do not experiment with UAV acquired images (Pérez et al., 2011). Ahmad et al. (2017), presented the result of investigating the accuracy of camera calibration at various UAV flying heights using a Sony NEX6 digital camera, 1.5m camera distance in the laboratory, 15m and 25m camera distances on the field with the most accurate camera parameters obtained from the 25m flying height which is also the optimum object distance. Fryskowska et al. (2016) experimented Image Master Calib, MATLAB - camera calibrator application and Agisoft Lens using Sony NEX 5 digital camera. The findings showed that Agisoft lens which automatically generates large numbers of tie points (Wierzbicki et al., 2015), based on Structure from Motion (SfM) algorithms (Westboy et al., 2012), using the Semi-Global Matching (SGM) algorithm (Hirschmüller, 2005) and Scale Invariant Feature Transform (SIFT) operator (Lowe, 2004), and also has the ability to perform self-calibration, gave optimal results together with the Image Master calib.

Thus, by extension, this research investigated the implication of not calibrating the camera, calibrating before the optimization of camera locations, calibrating during the optimization of camera locations and calibrating after the optimization of camera locations during the photogrammetric processing of UAV acquired images for DEM generation.

The imaging (study) area

The imaging area is part of the main campus of the Federal University of Technology (FUT), situated in Minna, Niger state, Nigeria. Geographically, the area lies between the boundaries of Northings 1055093.867mN and 1054587.539mN, and Eastings 217981.805mE and 220613.904mE. It can be better described as a stretch from the University's Computer Based Test Center to the School of Agriculture and Agricultural Technology (SAAT) complex, encompassing the School of Environmental Technology (SET) and the University's convocation square (see Figure 1), covering an approximate area of 11 hectares. The terrain configuration is relatively gentle and flat.

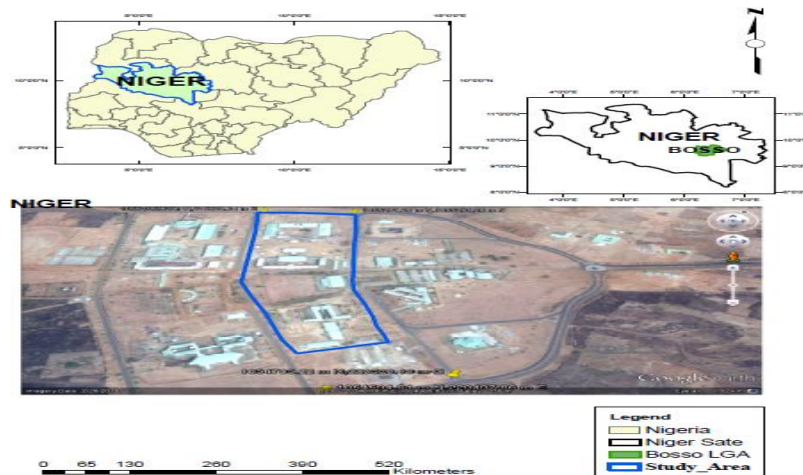


Figure 1: Map of the imaging area.

METHODOLOGY

The methodology adopted is subdivided into three major components viz: reconnaissance and flight planning, image data acquisition and image processing for DEM generation. During the reconnaissance, four ground control points (GCPs) were discovered to have been previously established within the study area (GPS3, GPS6, GPS7 and GPS 12) with the aid of a High-target DGPS receiver unit. In situ test was conducted on these GCPs and they were found to be in true position. Since these four GCPs are not sufficient, thirteen (13) more control points were also established using the same DGPS receivers, bringing the total number of GCPs to seventeen (17). The GCPs were pre-marked with reflective materials for easy identification before the flight mission. These control points were established for georeferencing, optimization of camera locations and assessment and validation of the DEMs' accuracy. DJI Phantom 3 professional; a rotary wing quadcopter UAV (see Figure 2 and Table 1 for its characteristics) was used for the image data acquisition in manual flight mode. The UAV was equipped with 12 megapixels, focal length of 2.8mm, and 4K resolution DJI FC300X camera. The camera has RGB band and operates in both manual and auto mode. The Quadcopter is powered by four (4) powerful brushless motors, with the arm span of 0.5m and weighs approximately 1.25kg including a payload of 0.75kg. To ensure a smooth and a safe flight mission, a pre-flight test was carried out to ascertain that every part of the UAV system functions properly and also, a flight plan (see figure 3) was designed to ensure total coverage of the entire imaging area and at the desired percentage overlap.

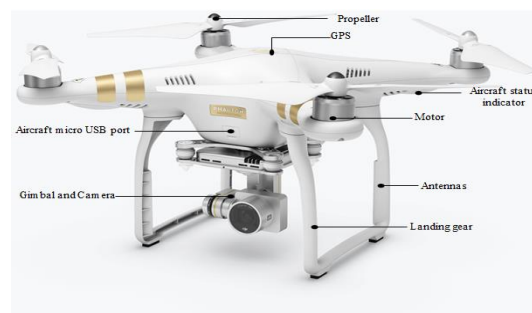


Figure 2: DJI Phantom 3 Professional Quadcopter UAV used for the study.

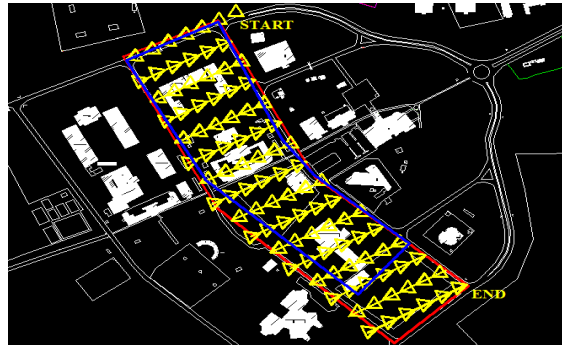


Figure 3: Flight map showing flight lines and direction of flight of the flight mission.

Table 1: The characteristics of the used UAV

AIRCRAFT	Specifications
Weight (including battery and propellers)	1280g
Diagonal size (Excluding propellers)	350mm
Max Ascent Speed	5m/s
Max Descent Speed	3m/s
Max Speed	16m/s (ATTI mode, no wind)
GPS Mode	GPS/GLONASS
Max Flight Time	Approximately 23 minutes
CAMERA	
Sensor	Sony EXMOR 1/2.3" Effective pixels: 12.4 M (total pixels: 12.76 M)
Lens	FOV 94° 20 mm f/2.8, focus at ∞
Photo	JPEG, DNG
GIMBAL	
Controllable Range	Pitch -90° to +30°
Stabilization	3-axis (pitch, roll, yaw)

(Source: <http://www.dji.com/phantom3pro>).

A total of 92 images were captured in manual mode at 15-20% overlap (sidelap and endlap), and these images were all utilized for the image processing. 15-20% overlap was adopted since the imaging area is relatively small and gentle, and also, a fixed flying height was adopted. Each of the images cover an approximate area of 4004.96m². The image processing procedure involved camera calibration, optimization of camera alignment, point cloud generation, dense cloud generation, auto-mosaicking and DSM/DEM generation, all performed using the Agisoft PhotoScan software. Figure 4 describes the process flow chart for generating the DEM, while the camera calibration parameters are presented in Table 2. These parameters include the computed principal distance or focal length (f) of the lens, parameters (x_p , y_p) which denote the coordinates of the center of projection of the image (principal point), and lens distortion coefficients (k_1 , k_2 , k_3 , p_1 , p_2) where the terms k_i represent coefficients of radial lens distortion and p_i terms represent coefficients of decentering distortion (Perez et al., 2011) caused by a lack of centering of lens elements. Radial and decentering distortions comprise the aberrations which affect the location of images (Fryer, 1996).

Table 2: Camera calibration parameters.

FC300X			
Resolution: 4000 x 2250	Focal Length: 2.8 mm	Pixel Size: 4e+06 x 2.25e+06 um	Precalibrated: No
Type:	Frame	Skew:	-0.53316
Fx:	2305.57	Cx:	1995.46
Fy:	2302.89	Cy:	1123.9
K1:	-0.130998	P1:	-0.00043097
K2:	0.107868	P2:	-0.000115319
K3:	-0.0157875	P3:	0

K4:	0	P4:	0
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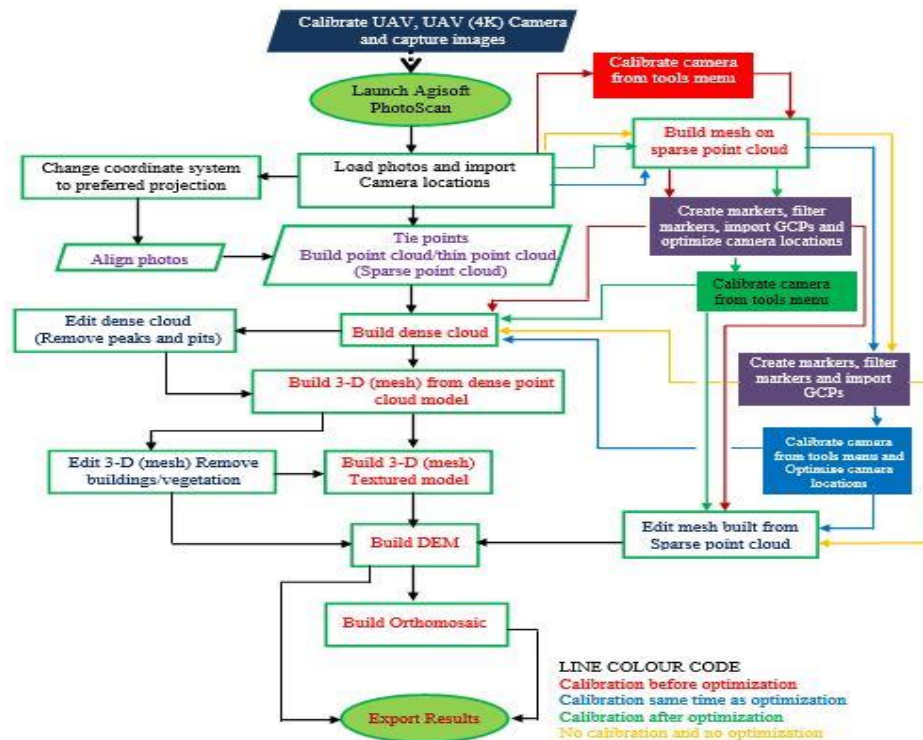


Figure 4: Step by step process adopted for the DEM generation using different camera calibration scenarios.

RESULTS AND DISCUSSION OF RESULTS

Figure 5 is a pictorial representation of the location of the GCPs carefully densified across the study area while Figure 6 presents the camera locations and the error estimates. The DEM produced in case scenario (1) is presented in Figure 7. The DEM shows that the highest point in the study area has an elevation value of 267m while the height value of the lowest point is 230m. Figure 8 depicts the DEM generated in case scenario (2). The model shows that the lowest height and highest height on the imaged area is 229m and 263m respectively. The DEM produced in case scenario (3) shows that the study area has a lowest height and highest height of 229m and 263m respectively as shown in figure 9, while figure 10 presents the DEM produced in case scenario (4). It shows that the imaged area has a lowest height and highest height of 229m and 262m respectively. In order to ascertain the accuracy of each of the generated DEMs, northings, eastings and height coordinates of the ground control points earlier established were extracted from each of the DEMs. The values of these extracted coordinates were compared with the coordinate values of the same points acquired during the control establishment (GCPs). The discrepancy was computed and used for the computation of the horizontal and vertical accuracy. Table 3 contains the result obtained from case scenario (1), Table 4 contains the result obtained from case scenario (2). The result obtained from case scenarios (3) and (4) are presented in Tables 5 and 6 respectively while Table 7 contains the summary of the horizontal and vertical accuracies obtained for the four different case scenarios.

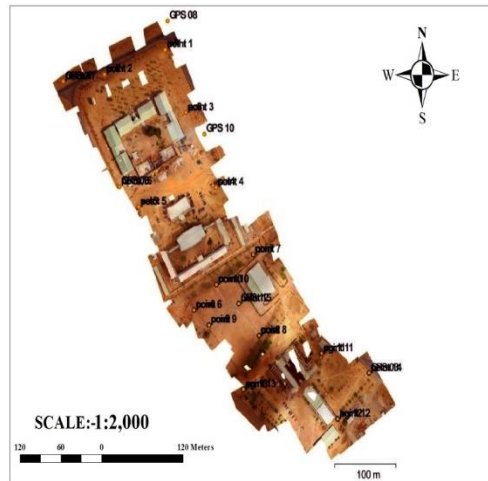


Figure 5: Location of GCPs on the imaging (study) area.

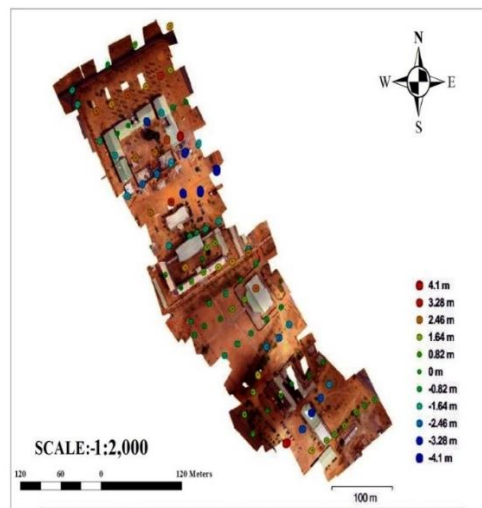


Figure 6: Camera locations and error estimates.

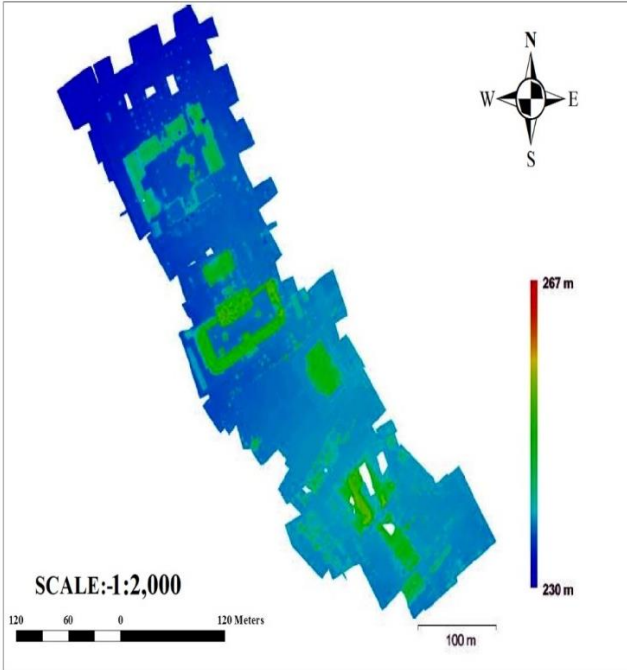


Figure 7: DEM produced without camera calibration and before optimization of camera alignment

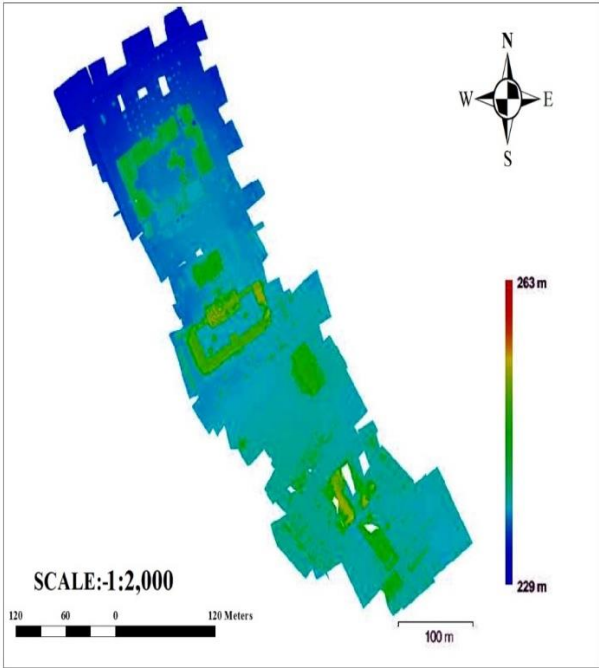


Figure 8: DEM generated by calibrating the camera optimization of camera alignment.

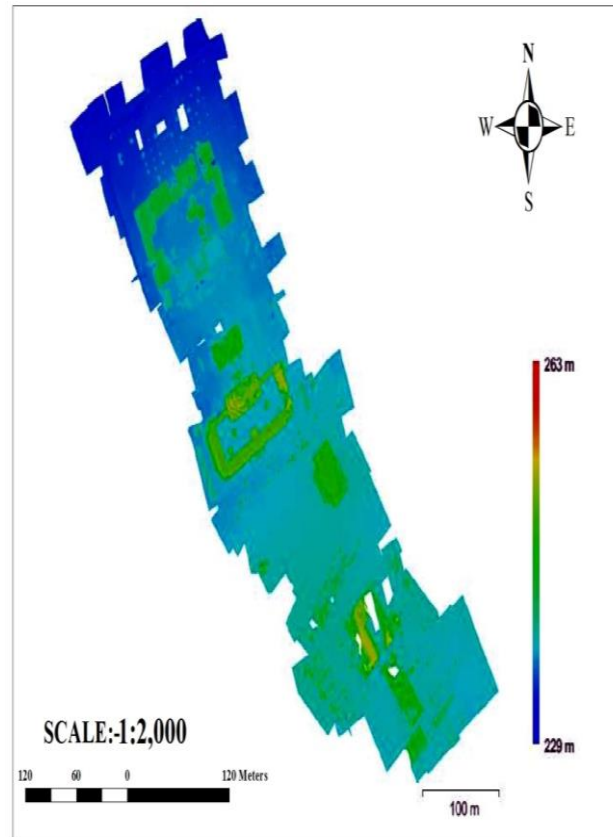
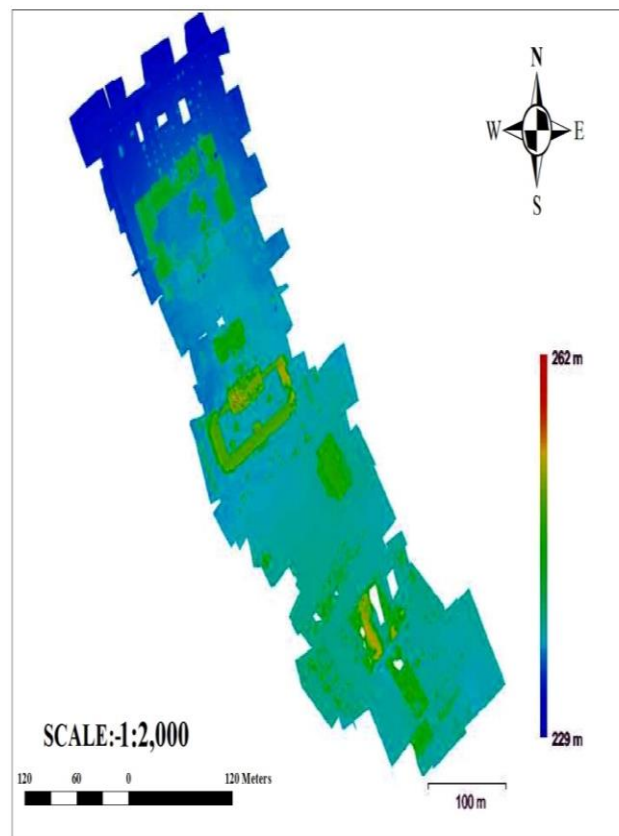


Figure 9: DEM generated by calibrating the camera during optimization of camera alignment.



10: DEM generated by calibrating the camera after optimization of camera alignment.

Table 3: Result obtained from DEM processed without camera calibration and optimization.

GCPs	DGPS ACQUIRED COORDINATES			DEM EXTRACTED COORDINATES (PROCESSING WITH NO CALIBRATION AND NO OPTIMIZATION)			COMPUTED DIFFERENCES		
	E (m)	N (m)	H (m)	N (m)	E (m)	H (m)	ΔE (m)	ΔN (m)	ΔH (m)
PT1	220147.938	1055132.392	233.390	220149.271	1055131.338	234.575	-1.333	1.054	-1.185
PT2	220048.540	1055099.532	232.729	220050.103	1055098.187	232.652	-1.563	1.345	0.077
PT3	220181.813	1055047.861	236.146	220183.007	1055046.715	236.542	-1.195	1.146	-0.396
PT4	220230.519	1054946.114	237.100	220232.181	1054945.658	237.363	-1.662	0.456	-0.263
PT5	220104.176	1054920.376	236.357	220105.935	1054919.438	235.874	-1.759	0.938	0.483
PT6	220194.365	1054780.450	237.455	220196.442	1054779.801	236.869	-2.078	0.649	0.586
PT7	220291.062	1054855.215	237.790	220292.918	1054854.724	237.978	-1.856	0.491	-0.188
PT8	220300.149	1054745.919	238.512	220302.442	1054745.493	238.268	-2.293	0.426	0.244
PT9	220218.684	1054759.701	237.919	220220.748	1054759.550	237.351	-2.064	0.151	0.568
PT10	220230.353	1054814.547	237.387	220232.794	1054814.063	237.080	-2.441	0.484	0.307
PT11	220402.310	1054721.078	238.324	220404.502	1054721.238	238.408	-2.192	-0.16	-0.084
PT12	220427.333	1054633.069	238.563	220430.871	1054632.610	238.468	-3.538	0.459	0.095
PT13	220274.221	1054673.378	238.876	220276.910	1054672.761	238.472	-2.689	0.617	0.404
GPS 03	220479.550	1054694.690	238.015	220481.978	1054694.835	238.080	-2.428	-0.15	-0.065
GPS 06	220072.920	1054947.700	235.968	220074.561	1054946.622	235.074	-1.641	1.078	0.894
GPS 07	219981.297	1055090.510	231.988	219982.847	1055088.865	231.471	-1.55	1.645	0.517
GPS 12	220267.286	1054789.050	238.188	220269.502	1054788.464	237.798	-2.216	0.586	0.39
SUM							-34.500	11.22	2.383

Table 4: Result obtained from DEM processed with calibration before optimization of camera alignment.

GCPs	DGPS ACQUIRED COORDINATES			DEM EXTRACTED COORDINATES (PROCESSING WITH CAMERA CALIBRATION BEFORE OPTIMIZATION OF CAMERA ALIGNMENT)			COMPUTED DIFFERENCES		
	E (m)	N (m)	H (m)	E (m)	N (m)	H (m)	ΔE (m)	ΔN (m)	ΔH (m)
PT1	220147.938	1055132.392	233.390	220147.943	1055132.413	233.857	-0.005	-0.02	-0.467
PT2	220048.540	1055099.532	232.729	220048.585	1055099.531	232.696	-0.045	1E-03	0.033
PT3	220181.813	1055047.861	236.146	220181.798	1055047.832	236.172	0.0145	0.029	-0.026
PT4	220230.519	1054946.114	237.100	220230.543	1054946.124	237.172	-0.024	-0.01	-0.072
PT5	220104.176	1054920.376	236.357	220104.163	1054920.410	236.415	0.013	-0.03	-0.058
PT6	220194.365	1054780.450	237.455	220194.215	1054780.288	237.47	0.1495	0.162	-0.015
PT7	220291.062	1054855.215	237.790	220290.986	1054855.225	237.763	0.0759	-0.01	0.027
PT8	220300.149	1054745.919	238.512	220300.182	1054745.948	238.471	-0.033	-0.03	0.041
PT9	220218.684	1054759.701	237.919	220218.592	1054759.963	237.856	0.0922	-0.26	0.063
PT10	220230.353	1054814.547	237.387	220230.624	1054814.565	237.377	-0.271	-0.02	0.01
PT11	220402.310	1054721.078	238.324	220402.235	1054721.158	238.306	0.0749	-0.08	0.018
PT12	220427.333	1054633.069	238.563	220427.425	1054632.989	238.547	-0.092	0.08	0.016
PT13	220274.221	1054673.378	238.876	220274.286	1054673.377	239.242	-0.065	1E-03	-0.366

GPS 03	220479.550	1054694.690	238.015	220479.473	1054694.658	237.848	0.077	0.032	0.167
GPS 06	220072.920	1054947.700	235.968	220072.870	1054947.670	235.671	0.05	0.03	0.297
GPS 07	219981.297	1055090.510	231.988	219981.280	1055090.494	231.921	0.0174	0.016	0.067
GPS 12	220267.286	1054789.050	238.188	220267.314	1054788.946	238.11	-0.028	0.104	0.078
SUM							0.0006	-0.01	-0.188

Table 5: Result obtained from DEM processed with calibration during optimization of camera alignment.

GCPs	DGPS ACQUIRED COORDINATES			DEM EXTRACTED COORDINATES (PROCESSING WITH CAMERA CALIBRATION DURING OPTIMIZATION)			COMPUTED DIFFERENCES		
	E (m)	N (m)	H(m)	E (m)	N (m)	H (m)	ΔE (m)	ΔN (m)	ΔH (m)
PT1	220147.938	1055132.392	233.390	220147.943	1055132.413	233.900	-0.005	-0.021	-0.51
PT2	220048.540	1055099.532	232.729	220048.585	1055099.531	232.673	-0.045	0.001	0.056
PT3	220181.813	1055047.861	236.146	220181.798	1055047.832	236.199	0.0145	0.029	-0.053
PT4	220230.519	1054946.114	237.100	220230.543	1054946.124	237.240	-0.024	-0.01	-0.14
PT5	220104.176	1054920.376	236.357	220104.163	1054920.410	236.527	0.013	-0.034	-0.17
PT6	220194.365	1054780.450	237.455	220194.215	1054780.288	237.439	0.1495	0.162	0.016
PT7	220291.062	1054855.215	237.790	220290.986	1054855.225	237.515	0.0759	-0.01	0.275
PT8	220300.149	1054745.919	238.512	220300.182	1054745.948	238.527	-0.033	-0.029	-0.015
PT9	220218.684	1054759.701	237.919	220218.592	1054759.963	237.893	0.0922	-0.262	0.026
PT10	220230.353	1054814.547	237.387	220230.624	1054814.565	237.328	-0.271	-0.018	0.059
PT11	220402.310	1054721.078	238.324	220402.235	1054721.158	238.301	0.0749	-0.08	0.023
PT12	220427.333	1054633.069	238.563	220427.425	1054632.989	238.287	-0.092	0.08	0.276
PT13	220274.221	1054673.378	238.876	220274.286	1054673.377	238.886	-0.065	0.001	-0.01
GPS 03	220479.550	1054694.690	238.015	220479.473	1054694.658	237.822	0.077	0.032	0.193
GPS 06	220072.920	1054947.700	235.968	220072.870	1054947.670	235.757	0.05	0.03	0.211
GPS 07	219981.297	1055090.510	231.988	219981.280	1055090.494	231.950	0.0174	0.016	0.038
GPS 12	220267.286	1054789.050	238.188	220267.314	1054788.946	238.100	-0.028	0.104	0.088
SUM							0.0006	-0.009	0.362

Table 6: Result obtained from DEM processed with calibration after optimization of camera alignment.

GCPs	DGPS ACQUIRED COORDINATES			DEM EXTRACTED COORDINATES (PROCESSING WITH CAMERA CALIBRATION AFTER OPTIMIZATION)			COMPUTED DIFFERENCES		
	E (m)	N (m)	H(m)	E (m)	N (m)	H (m)	ΔE (m)	ΔN (m)	ΔH (m)
PT1	220147.938	1055132.392	233.390	220147.943	1055132.413	233.948	-0.0051	-0.021	-0.558
PT2	220048.540	1055099.532	232.729	220048.585	1055099.531	232.684	-0.0447	0.001	0.0453
PT3	220181.813	1055047.861	236.146	220181.798	1055047.832	236.164	0.0145	0.029	-0.018
PT4	220230.519	1054946.114	237.100	220230.543	1054946.124	237.195	-0.0242	-0.01	-0.0954
PT5	220104.176	1054920.376	236.357	220104.163	1054920.410	236.560	0.013	-0.034	-0.2034
PT6	220194.365	1054780.450	237.455	220194.215	1054780.288	237.415	0.1495	0.162	0.0402
PT7	220291.062	1054855.215	237.790	220290.986	1054855.225	237.559	0.0759	-0.01	0.2308
PT8	220300.149	1054745.919	238.512	220300.182	1054745.948	238.520	-0.0333	-0.029	-0.0082

PT9	220218.684	1054759.701	237.919	220218.592	1054759.963	237.906	0.0922	-0.262	0.0134
PT10	220230.353	1054814.547	237.387	220230.624	1054814.565	237.399	-0.2713	-0.018	-0.0122
PT11	220402.310	1054721.078	238.324	220402.235	1054721.158	238.306	0.0749	-0.08	0.0176
PT12	220427.333	1054633.069	238.563	220427.425	1054632.989	238.293	-0.0924	0.08	0.2696
PT13	220274.221	1054673.378	238.876	220274.286	1054673.377	238.856	-0.0647	0.001	0.0201
GPS 03	220479.550	1054694.690	238.015	220479.473	1054694.658	237.794	0.077	0.032	0.221
GPS 06	220072.920	1054947.700	235.968	220072.870	1054947.670	235.453	0.05	0.03	0.5146
GPS 07	219981.297	1055090.510	231.988	219981.280	1055090.494	231.953	0.0174	0.016	0.0354
GPS 12	220267.286	1054789.050	238.188	220267.314	1054788.946	238.108	-0.0281	0.104	0.0798
SUM							0.0006	-0.009	0.5926

Table 7: Analysis of the RMSE, horizontal and vertical accuracies obtained for the four different case scenarios.

S/N	CONDITIONS	RMSE _r	RMSE _z	HORIZONTAL ACCURACY	VERTICAL ACCURACY
1	No camera calibration	8.79825854	0.57786538	15.22802588	1.132616145
2	Calibration before optimization of camera Alignment	0.002187666	0.045693712	0.003786412	0.089559675
3	Calibration during optimization of camera alignment	0.002187666	0.087700882	0.003786412	0.171893729
4	Calibration after optimization of camera alignment	0.002187666	0.143726611	0.003786412	0.281704158

The Root Mean Square Error (RMSE) was calculated using the formula given in equation (1):

$$RMSE = \sqrt{\frac{\sum(N_i - N_j)^2}{n}} \quad (1)$$

Where N_i is observed values, N_j is reference values and n is number of points.

The horizontal and vertical accuracy was computed using the National Standard for Spatial Data Accuracy (NSSDA) method for map accuracy which is given as equations (2) and (3):

$$\text{Horizontal Accuracy} = 1.7308 \times RMSE_r \quad (2)$$

$$\text{Vertical Accuracy} = 1.96 \times RMSE_z \quad (3)$$

Where $RMSE_r$ and $RMSE_z$ are the Root mean square errors of the horizontal and vertical discrepancy respectively.

From the results obtained, it was discovered that case scenario (1) gave coordinate values that are of widest variance when compared with the coordinate values of DPGS observation. Change in Eastings (ΔE) of -34.500m, Changes in Northings (ΔN) of 11.22m and Height (ΔH) of 2.383m were observed and recorded, while case scenario (2) gave the lowest discrepancy value with ΔE of 0.0006m, ΔN of -0.01m and ΔH of -0.188m. Case scenarios (3) and (4) gave ΔE and ΔN values of 0.0006m and -0.009m respectively with a ΔH value of 0.362m for case scenario (3) and ΔH value of 0.5926m for case scenario (4). Also, a horizontal accuracy of 15.228 and vertical accuracy of 1.133 was obtained from the analysis of case scenario (1), horizontal accuracy of 0.003 was obtained from case scenarios (2), (3) and (4) respectively (see Table 7). This implies that the timing of camera calibration (before during or after the optimization of camera alignment) has no implication on the horizontal accuracy of the generated DEMs. Vertical accuracy of 0.089 was obtained from case scenario (2), while 0.171 was obtained from case scenario (3) and 0.281 obtained from case scenario (4). This shows that the timing of camera calibration (before, during or after the optimization of camera alignment) has a great impact on the obtainable vertical accuracy of DEMs even though the horizontal reliability of the generated DEM seems immune to this. From our results, the vertical accuracy obtained from case scenario (2) where the camera was calibrated before the optimization of camera alignment

was the lowest (closest to zero) and thus, most accurate of the four different case scenarios. This was closely followed by case scenario (3) and then case scenario (4).

CONCLUSIONS

It was discovered from the obtained results that the coordinates extracted from the DEM generated via a process where the camera was calibrated before the optimization of camera alignment were closer to the coordinates obtained from the field observation using High-target DGPS receivers when compared to the other three case scenarios. This method gave a horizontal accuracy of 0.003 and a vertical accuracy of 0.089. It is thus necessary to ensure that the operation of camera calibration is performed before the optimization of camera alignment when processing image data of this nature for DEM production using the Agisoft photoscan digital photogrammetric software. Also, it was discovered that the same horizontal accuracy was obtained from the processes where the camera was calibrated before, during and after the optimization of camera alignment which means that the timing of camera calibration has little or no effect on the horizontal or planimetric integrity of the produced DEMs. Finally, the worst (most inaccurate) result was obtained from case scenario (1) which means that camera calibration is highly important in the production of DEMs from UAV acquired overlapping images.

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REFERENCES

- Ahmad, R. Y., Mohd, F. M. A., Khairulnizam, M. I., Zulkepli, M., and Albert, K. C. (2017) Camera Calibration Accuracy at different UAV Flying Heights. *The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences*, Volume XLII-2/W3, 2017 3D Virtual Reconstruction and Visualization of Complex Architectures, 1–3 March 2017, Nafplio, Greece, pp. 595-600
- Ajayi, O. G., Salubi, A. A., Angbas, A. F., and Odigure, M. G. (2017) Generation of accurate digital elevation models from UAV acquired low percentage overlapping images. *International Journal of Remote Sensing*. 8-10(38), pp. 3113-3134.
- Anuar, A. (2011) Digital Mapping Using Low Altitude UAV. *Pertanika Journal of Science and Technology*. 19 (S), pp. 51-58.
- Birute, R., Tautvydas, B., Silvija, G., Edita, J., and Vladislovas C. A. (2014) Photogrammetric Processing of UAV Imagery: Checking DTM. *The 9th International Conference “Environmental Engineering” 22–23 May 2014, Vilnius, Lithuania*. Available online at <http://enviro.vgtu.lt>
- Bolstad, P. V. and Stowe, T. (1994) An Evaluation of DEM Accuracy: Elevation, Slope and Aspect, *Photogrammetric Engineering and Remote Sensing*. 60(11), pp. 1327-1332.
- Douskos V., Kalisperakis I. and Karras G. 2007. Automatic calibration of digital cameras using planar chessboard patterns. *8th Conf. Opt. 3-D Meas. Techn., Wichmann*, vol. I, pp. 132-140.
- Eisenbeiss, H., (2009). *UAV Photogrammetry*, PhD Thesis, University of Technology Dresden, Zurich, Germany.
- Fonstad, M.A., Dietrich, J.T., Courville, B.C., Jensen, J.L. and Carbonneau, P.E. (2013) Topographic structure from motion: a new development in photogrammetric measurement. *Earth Surf. Process. Landforms*. 38, pp. 421–430.
- Fryer, J.G. (1996) Camera calibration. In: *Close Range Photogrammetry and Machine Vision*. K.B. Atkinson (Eds), Whittler Publishing, Caithness, pp. 156-179.

- Fryskowska, A., Kedzierski, M., Grochala, A., and Braula, A. (2016) Calibration of Low-Cost RGB And NIR UAV Cameras. The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences, Volume XLI-B1, 2016 XXIII ISPRS Congress, 12–19 July 2016, Prague, Czech Republic, pp. 817-821.
- Hirschmüller, H. (2005) Accurate and Efficient Stereo Processing by Semi-Global Matching and Mutual Information, IEEE Conference on Computer Vision and Pattern Recognition, June 2005, San Diego, CA, USA, Volume 2, pp. 807-814.
- Isioye, O. A and Jobin, P. (2011) An Assessment of Digital Elevation Models (DEMs) From Different Spatial Data Sources. In Proceedings of the FIG Working Week 2011, Bridging the Gap between Cultures Marrakech, Morocco, 18-22 May 2011.
- Lowe, D. (2004) Distinctive Image Features from Scale-Invariant Keypoints. International Journal of Computer Vision, 60(2), pp. 91-110.
- Norhadija, D., Nurul, F.A.H., Wani, S.U., Noor, A.B.M.A., and Anuar, A. (2013) Light Weight Rotary-Wing UAV for Large Scale Mapping Applications. Asia Geospatial Forum, 24-26 September 2013, Kuala Lumpur, Malaysia.
- Pérez, M., Agüera, F., and Carvajal, F. (2011) Digital camera calibration using images taken from an unmanned aerial vehicle. International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences, Vol. XXXVIII-1/C22 UAV-g 2011, Conference on Unmanned Aerial Vehicle in Geomatics, Zurich, Switzerland, pp. 1-5
- Sammartano, G. and Spanò, A. (2016) DEM Generation based on UAV Photogrammetry Data in Critical Areas. In Proceedings of the 2nd International Conference on Geographical Information Systems Theory, Applications and Management (GISTAM 2016), pp. 92-98
- Sulebak, J. R. (2000) Applications of Digital Elevation Models. DYNAMAP " White Paper", 2000, pp.1-11
- Toz, G. and Erdogan, M. (2008) DEM (digital elevation model) production and accuracy modelling of DEMs from 1:35.000 scale aerial photographs. The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences. Vol. XXXVII. Part B1. Beijing 2008, pp. 775-780
- Weng, J., Cohen, P., and M. H. 1992. Camera calibration with distortion models and accuracy evaluation. IEEE Trans, Pattern Analysis and Machine Intelligence, vol.14, no.10, October 1992, pp. 965-980.
- Westboy, M. J., Brasington, J., Glasser, N. F., Hambrey, M. J., and Reynolds, J. M. (2012) Structure-from-Motion Photogrammetry: A low-cost, effective tool for geoscience applications. Geomorphology, 179, pp. 300-314.
- Wierzbicki, D., Kedzierski, M., and Fryskowska, A. (2015) Assessment of the influence of UAV image quality on the orthophoto production. The International Archives of the Photogrammetry, Remote Sensing and Spatial Information Sciences, Volume XL-1/W4, 2015.
- Zhang, W., Jiang, T. and Han, M. 2010. Digital camera calibration method based on PhotoModeler. 3rd International Congress on Image and Signal Processing (CISP2010), pp. 1235-1238.
- Zhang, Z. (2004) Camera Calibration. In G. Medioni and S. B. Kang (eds.), Emerging Topics in Computer Vision, Prentice Hall Professional Technical Reference, Chapter 2, 4-43.

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